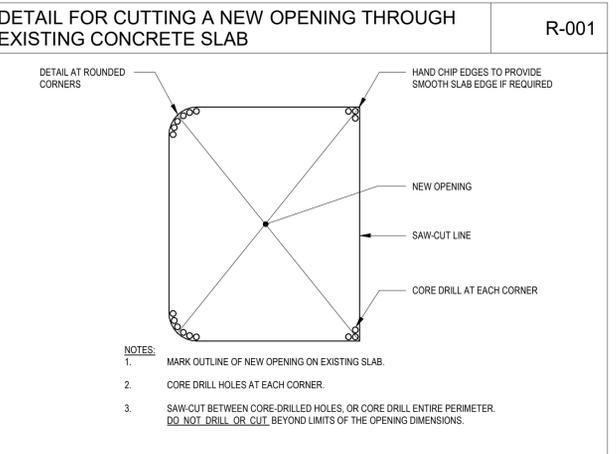


ARCH D - 24"x36" - 6 (Dimm) 9/4mm (rounded)

GENERAL NOTES		GN-001CS
1	<b>GENERAL</b>	
1.1	CHECK DIMENSIONS ON STRUCTURAL DRAWINGS AGAINST ARCHITECTURAL DRAWINGS AND EXISTING SITE CONDITIONS. REPORT INCONSISTENCIES TO CONSULTANT BEFORE PROCEEDING WITH THE WORK.	
1.2	READ DRAWINGS IN CONJUNCTION WITH SPECIFICATIONS.	
1.3	DO NOT EXCEED DURING CONSTRUCTION DESIGN LOADS SHOWN ON PLANS REDUCED AS NECESSARY UNTIL MATERIALS REACH DESIGN STRENGTH.	
1.4	DO NOT SCALE DRAWINGS.	
1.5	DIMENSIONS ARE IN MILLIMETRES UNLESS NOTED OTHERWISE. ELEVATIONS ARE IN METRES UNLESS NOTED OTHERWISE.	
2	<b>DESIGN</b>	
2.1	ALL REFERENCED STANDARDS LISTED SHALL BE THE CURRENT PUBLISHED EDITION OR THE EDITION REFERENCED BY THE APPLICABLE BUILDING CODE IN FORCE AT THE DATE NOTED ON THE STRUCTURAL DRAWINGS FOR THE BUILDING PERMIT APPLICATION.	
2.2	DESIGN IS IN ACCORDANCE WITH THE ONTARIO BUILDING CODE.	
2.3	DESIGN STANDARDS	
2.3.1	CONCRETE MEMBERS ARE DESIGNED IN ACCORDANCE WITH CSA STANDARD A23.3.	
2.3.2	STRUCTURAL STEEL IS DESIGNED IN ACCORDANCE WITH CSA STANDARD S16.	
3	<b>MATERIALS</b>	
3.1	CONCRETE: SEE SCHEDULE OF CONCRETE PROPERTIES AND SPECIFICATION.	
3.2	STRUCTURAL STEEL: UNLESS NOTED OTHERWISE TO CSA G40.21/G40.21 OR ASTM STANDARD A992/A992M OR ASTM A572.	
3.3	W AND S SHAPES: ASTM A992 OR ASTM A572 GRADE 50. FY=345 MPa	
3.3.1	PLATES: CSA G40.21 GRADE 350 W	
3.3.2	CHANNELS AND ANGLES: CSA G40.21 GRADE 300W	
3.3.3	HOLLOW STRUCTURAL SECTIONS: CSA G40.21 GRADE 350W CLASS C OR ASTM STANDARD A1065	
3.3.4	ANCHOR RODS: ASTM F1554 GRADE 36, 55, 105 (SELECT GRADE)	
3.3.5	REINFORCING STEEL: TO CONFORM TO CSA G30.18 GRADE 400W UNLESS NOTED OTHERWISE	
3.3.6	REINFORCING BAR AREAS ARE: 100, 200, 300, 500, 700, 1000, 1500 AND 2500 SQ. MM	
3.3.7	FOR BAR DESIGNATIONS 10M, 15M, 20M, 25M, 30M, 35M, 45M AND 55M RESPECTIVELY.	
3.3.8	STRENGTH:	
3.3.9	DEFORCED REINFORCING BARS: 400 MPa	
3.3.10	WELDED WIRE FABRIC: 440 MPa	
4	<b>STRUCTURAL ADEQUACY OF EXISTING BUILDING</b>	
4.1	UPON COMPLETION OF THE STRUCTURAL MODIFICATIONS AND UPGRADES DETAILED ON THE DRAWINGS AND SPECIFICATIONS PREPARED FOR THIS PROJECT, THE EXISTING REINFORCED STRUCTURE AND SUPPORTING MEMBERS WILL HAVE ADEQUATE CAPACITY TO SUPPORT THE ADDITIONAL LOADS IMPOSED BY THE PROPOSED RENOVATIONS.	
4.2	WHERE SUPPLEMENTARY REINFORCING HAS NOT BEEN SPECIFIED TO EXISTING STRUCTURAL MEMBERS, THESE MEMBERS ARE EITHER NOT AFFECTED BY THE PROPOSED RENOVATIONS OR THEY HAVE BEEN ANALYZED AND CONFIRMED TO BE ADEQUATE AS-IS.	

DESIGN LOAD		GN-002CS
1.	UNIT FLOOR AND ROOF LOADINGS, SOIL BEARING PRESSURES AND FOUNDATION LOADS GIVEN ON PLANS ARE UNFACTORED. MEMBER FORCES GIVEN ON DRAWINGS ARE FACTORED.	
2.	GRAVITY LOADS: SUPERIMPOSED DEAD LOADS AND LIVE LOADS ARE GIVEN ON PLANS.	
3.	SNOW LOAD PARAMETERS, OBC 2012 - NORTH YORK, ONTARIO	
	$S_s = 1.2$	
	$S_r = 0.4$	
	$W_s = 1.25$	
	$W_{SL} = 0.9$	
4.	RAIN LOAD PARAMETER, OBC 2012 - NORTH YORK, ONTARIO	
	ONE DAY RAINFALL = 108 mm	
5.	WIND LOAD PARAMETERS, OBC 2012 - NORTH YORK, ONTARIO	
	$q(110) = 0.34 \text{ kPa}$	
	$q(150) = 0.44 \text{ kPa}$	
	$W_s = 1.25$	
	$W_{SL} = 0.75$	
6.	SEISMIC LOAD PARAMETERS, OBC - NORTH YORK, ONTARIO	
	$S_a(0.2) = 0.195$	$S_a(0.0) = 0.067$
	$S_a(0.5) = 0.107$	$S_a(1.0) = 0.027$
	$S_a(1.0) = 0.056$	$PGA = 0.125$
	$S_a(2.0) = 0.0280$	$PGV = 0.083$
	SITE CLASSIFICATION = D	
	$I_e = 1.5$	
	$F_a = 1.2$	
	$F = 1.51$	
	$I_e F_a S_a(0.2) = 0.35$	



STRUCTURAL STEEL NOTES		GN-006CS
1	<b>GENERAL</b>	
1.1	THE FOLLOWING REFERENCE STANDARDS SHALL GOVERN THE WORK OF THIS SECTION:	
1.1.1	ASTM A108, SPECIFICATION FOR STEEL BAR, CARBON AND ALLOY, COLD FINISHED	
1.1.2	CSA S16, DESIGN OF STEEL STRUCTURES	
1.1.3	CSA S18, NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS	
1.1.4	CSA W47.1, CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL	
1.1.5	CSA W59, WELDED STEEL CONSTRUCTION	
1.1.6	CSA W178.1, CERTIFICATION OF WELDING INSPECTION ORGANIZATIONS	
1.1.7	CSA W178.2, CERTIFICATION OF WELDING INSPECTORS	
1.2	DESIGN OF CONNECTIONS SHALL BE BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO	
1.3	<b>DESIGN CRITERIA</b>	
1.3.1	AXIAL LOADED MEMBERS THAT MEET AT A JOINT SHALL HAVE THEIR CENTROIDAL AXES INTERSECT AT A COMMON POINT UNLESS SHOWN OTHERWISE.	
1.3.2	DESIGN AND DETAIL ALL CONNECTIONS AS FLEXIBLE EXCEPT WHERE NOTED OTHERWISE ON THE DRAWINGS. CONNECTIONS MAY BE WELDED OR BOLTED.	
1.3.3	PROVIDE CONNECTIONS ADEQUATE TO RESIST REACTION OF BEAM WHEN IT IS LOADED TO MAXIMUM FLEXURAL CAPACITY UNDER UNIFORMLY DISTRIBUTED LOAD, UNLESS REACTION OR CONNECTION DETAIL IS SHOWN ON DRAWINGS. FOR COMPOSITE BEAM CONSTRUCTION, USE FLEXURAL CAPACITY OF COMPOSITE SECTION BASED ON 100 PERCENT SHEAR CONNECTION OF BEAM TO SLAB.	
1.3.4	WHERE MOMENT CONNECTIONS ARE CALLED FOR BUT DESIGN FORCES ARE NOT INDICATED, DESIGN MOMENT CONNECTION FOR THE FULL MOMENT CAPACITY OF THE WEAKER MEMBER JOINED.	
1.3.5	FOR BOLTED CONNECTIONS USE SNUG TIGHT HIGH STRENGTH BOLTS, ASTM F125/F3125M (A325 OR A490) EXCEPT USE PRETENSIONED HIGH STRENGTH BOLTS IN LOCATIONS SPECIFIED IN CSA-S16 CLAUSE 22.22	
1	SUP-CRITICAL CONNECTIONS WHERE SUPRACE CANNOT BE TOLERATED:	
2	SHEAR CONNECTIONS PROPORTIONED IN ACCORDANCE WITH SEISMIC REQUIREMENTS;	
3	ALL ELEMENTS RESISTING CRANE LOADS;	
4	CONNECTIONS SUBJECT TO IMPACT OR CYCLIC LOADING;	
5	CONNECTIONS WHERE THE BOLTS ARE SUBJECT TO TENSILE LOADING;	
6	CONNECTIONS USING OVERSIZE OR LONG SLOTTED HOLES (UNLESS SPECIFICALLY DESIGNED TO ACCOMMODATE MOVEMENT).	
1.3.6	PROVIDE CONNECTIONS ON MEMBERS THAT ARE PART OF THE LATERAL LOAD RESISTING SYSTEM ADEQUATE TO RESIST FORCES SHOWN ON DRAWINGS, WHERE SEISMIC DESIGN GOVERNS, THE FORCES HAVE BEEN ADJUST TO MEET THE REQUIREMENTS OF CLAUSE 27.	
1.4	<b>SUBMITTALS</b>	
1.4.1	SUBMIT STRUCTURAL SHOP DRAWINGS	
1	EACH SHOP DRAWING SUBMITTED SHALL BEAR THE SIGNATURE AND SEAL OF THE PROFESSIONAL ENGINEER RESPONSIBLE FOR CONNECTION DESIGN.	
2	<b>PRODUCTS</b>	
2.1	<b>MATERIAL</b>	
2.1.1	PROVIDE NEW MATERIALS IN ACCORDANCE WITH REFERENCE STANDARDS, OF STRENGTH AND QUALITY NOTED IN GENERAL NOTES.	
2.1.2	STUDS: ASTM A108	
2.1.3	GALVANIZING: ASTM A123A123M, STANDARD SPECIFICATION FOR ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS	
2.1.4	GALVANIZING: HOT-DIP TO ASTM A153 / A153M-16 STANDARD SPECIFICATION FOR ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE	
2.1.5	<b>PAINT:</b>	
1	INTERIOR: SHOP COAT FOR STEEL THAT WILL NOT RECEIVE A FINISH COAT. TO CISC/CPMA STANDARD 1-73A. A QUICK-DRYING ONE-COAT PAINT FOR USE ON STRUCTURAL STEEL.	
2	INTERIOR: PRIME PAINT. TO MEET THE REQUIREMENTS OF CISC/CPMA STANDARD 2-75, A QUICK-DRYING PRIMER FOR USE ON STRUCTURAL STEEL.	
3	EXTERIOR: ZINC-RICH PAINT READY MIX TO SSPC-PAIN 20 STANDARD	
3	<b>EXECUTION</b>	
3.1	PROVIDE WELDED STIFFENER PLATES MINIMUM 10 mm THICK ON BOTH SIDES OF WEB OF BEAMS AT POINTS OF CONCENTRATED LOADS INCLUDING BEAMS SUPPORTING COLUMNS OR BEAMS SUPPORTED ON TOP OF COLUMNS.	
3.2	ALL EXPOSED WEELS SHALL BE CONTINUOUS AND GROUND SMOOTH.	
3.3	PROVIDE STRUCTURAL STEEL FOR LATERAL SUPPORT OF MASONRY WALLS.	
3.4	CLEAN STEEL IN ACCORDANCE WITH PAINT SYSTEM SPECIFIED. ZINC-RICH PAINT REQUIRES CLEANING TO SSPC-SP6, COMMERCIAL BLAST CLEANING.	
3.5	<b>PAINTING:</b>	
3.5.1	PAINT INTERIOR STEEL SURFACES WITH INTERIOR PAINT SPECIFIED.	
3.5.2	PAINT EXTERIOR STEEL SURFACES WITH EXTERIOR PAINT SPECIFIED.	
3.5.3	DO NOT PAINT:	
1	SURFACES AND EDGES WITHIN 50 mm OF FIELD WELDS	
2	SURFACES ENCASED IN OR IN CONTACT WITH CONCRETE	
3	SURFACES TO BE SPRAY FIREPROOFED.	
3.5.4	AFTER ERECTION IS COMPLETE GIVE ONE COAT TOUCH-UP PAINT TO FIELD BOLTS, FIELD CONNECTIONS, BURNT AREAS AND DAMAGED AREAS. USE SAME PAINT AS SHOP PAINT.	
3.6	GALVANIZE UNITS BRICK SUPPORT ANGLES, ARCHITECTURAL BLOCK SUPPORT ANGLES AND OTHER MEMBERS INDICATED AS GALVANIZED ON DRAWINGS AFTER SHOP WELDING IS COMPLETE.	
3.7	COMPLY WITH THE REQUIREMENTS OF REFERENCE STANDARDS AND REQUIREMENTS OF REGULATORY AUTHORITIES IN ERECTION OF STRUCTURAL STEEL.	
3.8	PROVIDE MINIMUM BEARING FOR ALL STEEL BEAMS:	
3.8.1	200 mm (8") ON CONCRETE AND MASONRY	
3.8.2	100 mm (4") ON STEEL	
4	<b>FIELD QUALITY CONTROL</b>	
4.1	<b>INSPECTION AND TESTING COMPANY SHALL PERFORM:</b>	
4.1.1	INSPECTION OF ERECTION INCLUDING PLACING, PLUMBING AND LEVELLING;	
4.1.2	INSPECTION OF BOLTED CONNECTIONS INCLUDING VERIFICATION OF BOLT GRADE AND IF SNUG TIGHT AND PRE-TENSIONED BOLTS HAVE BEEN USED APPROPRIATELY;	
4.1.3	INSPECTION OF WELDED JOINTS;	
4.1.4	GENERAL INSPECTION OF FIELD CUTTING AND ALTERATIONS;	
4.1.5	GENERAL INSPECTION OF COATING TOUCH-UP.	

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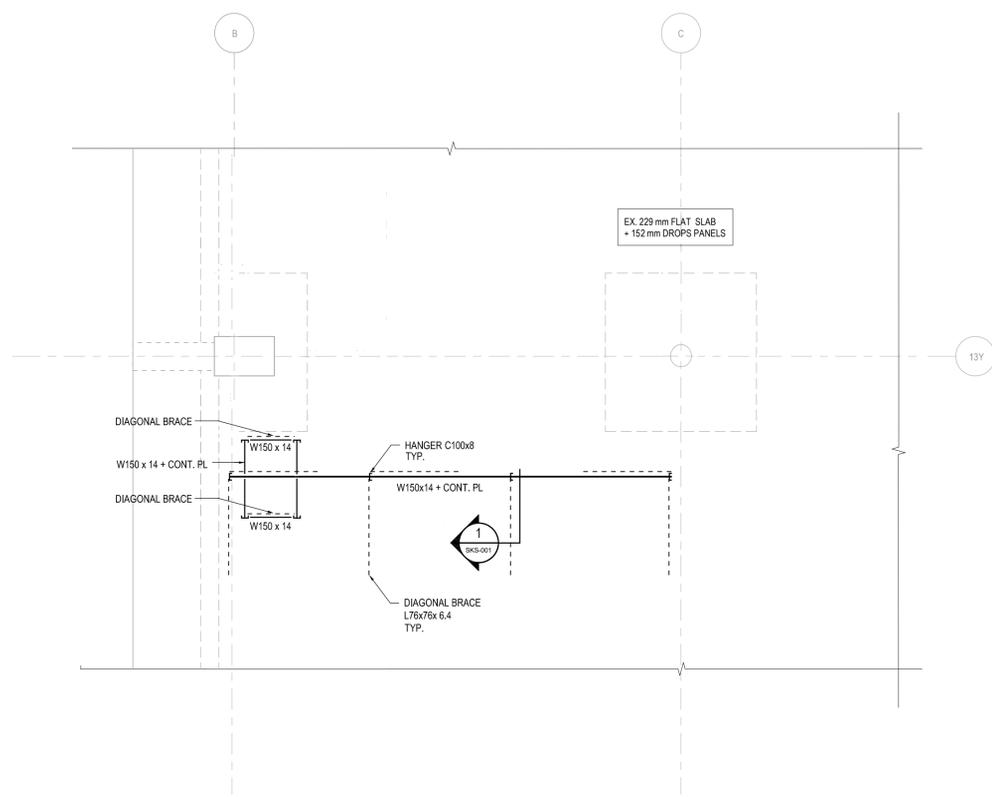
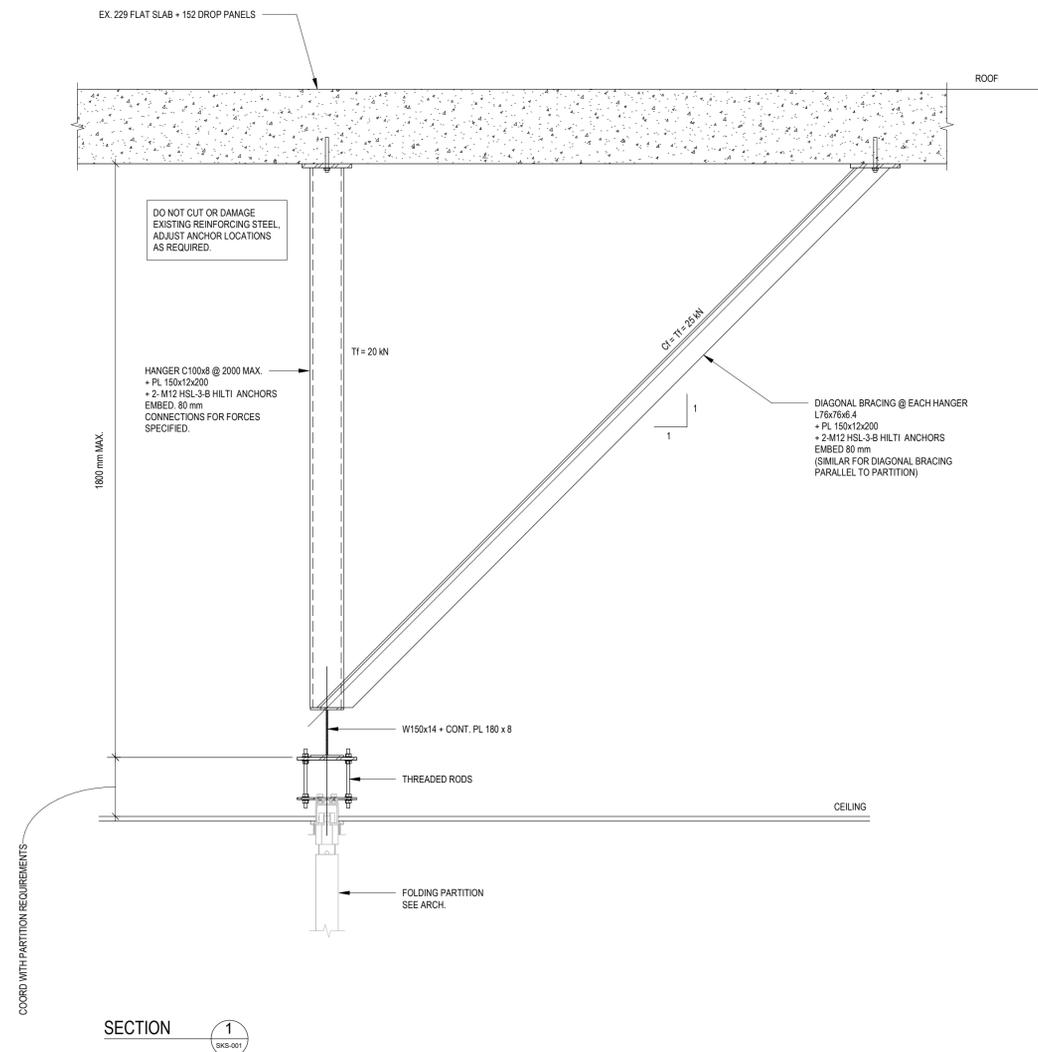
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NON-STRUCTURAL ELEMENTS		GN-011CS
1.	"NON-STRUCTURAL" OR "SECONDARY STRUCTURAL" ELEMENTS ARE NOT PART OF THE STRUCTURAL DESIGN AND THEREFORE NOT SHOWN ON THESE DRAWINGS. SUCH ELEMENTS ARE DESIGNED, DETAILED AND REVIEWED IN THE FIELD BY OTHERS. WHERE STRUCTURAL ENGINEERING RESPONSIBILITY IS REQUIRED FOR THESE ELEMENTS, THIS MUST BE PROVIDED BY SPECIALTY STRUCTURAL ENGINEERS, WHO MUST ALSO PROVIDE ANY DOCUMENTATION REQUIRED BY BUILDING PERMIT AUTHORITIES.	
2.	EXAMPLES OF NON-STRUCTURAL ELEMENTS INCLUDE, BUT ARE NOT LIMITED TO:	
a.	ARCHITECTURAL COMPONENTS SUCH AS GUARDRAILS, HANDRAILS, MISCELLANEOUS STEEL STAIRS, FLAG POLES, CANOPIES, CEILING, MILLWORK, ETC.	
b.	LANDSCAPE ELEMENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, ETC.	
c.	CLADDING, GLAZING, WINDOW MULLIONS, INTERIOR STUD WALLS AND EXTERIOR STUD WALLS.	
d.	ARCHITECTURAL PRECAST, PRECAST CLADDING.	
e.	SKYLIGHTS.	
f.	MECHANICAL AND ELECTRICAL EQUIPMENT, COMPONENTS, AND THEIR ATTACHMENT DETAILS, CONNECTIONS DESIGNED SEISMIC RESTRAINT ACCORDING TO OBC 2012 CLAUSE 4.1.8.	
g.	WINDOW WASHING EQUIPMENT, FALL ARREST ANCHORS AND THEIR ATTACHMENTS.	
h.	ESCALATORS, ELEVATORS, AND CONVEYING SYSTEMS.	
i.	GLASS BLOCK AND ITS ATTACHMENTS.	
j.	BRICK OR BLOCK VENEERS AND THEIR ATTACHMENTS.	
k.	NON-LOAD BEARING MASONRY.	
l.	NON-STRUCTURAL CONCRETE TOPPING.	
3.	SUBMIT SHOP DRAWINGS FOR NON-STRUCTURAL ELEMENTS WHICH MAY AFFECT THE PRIMARY STRUCTURAL SYSTEM TO EIP. THESE DRAWINGS WILL BE REVIEWED ONLY FOR THE LOAD EFFECT OF THE ELEMENT ON THE PRIMARY STRUCTURAL SYSTEM.	

CONCRETE ANCHORS, INSERTS, BOLTS		GN-012CS
1	<b>GENERAL</b>	
1.1	THE FOLLOWING REFERENCE STANDARDS SHALL GOVERN THE WORK OF THIS SECTION:	
1.1.1	CSA A23.3, DESIGN OF CONCRETE STRUCTURES	
2	<b>PRODUCTS</b>	
2.1	TORQUE CONTROLLED EXPANSION ANCHORS	
2.1.1	EXPANSION ANCHOR: PROVIDE EXPANSION ANCHORS OF SIZE SHOWN ON DRAWINGS, INCLUDING MATCHING NUTS AND WASHERS.	
1	FOR DRY LOCATIONS:	
	KWIK BOLT T2 CARBON STEEL ZINC PLATED BY HILTI (CANADA) CORPORATION, MISSISSAUGA, ONTARIO	
2	FOR WET OR HIGH HUMIDITY LOCATIONS OR LOCATIONS EXTERIOR TO THE CONDITIONED BUILDING	
ENVELOPE:		
	KWIK BOLT T2 TYPE 304 STAINLESS STEEL, BY HILTI (CANADA) CORPORATION, MISSISSAUGA, ONTARIO	
3	FOR LOCATIONS EXPOSED TO CHLORIDES OR OTHER CORROSIVE MATERIALS:	
	KWIK BOLT T2 TYPE 316 STAINLESS STEEL, BY HILTI (CANADA) CORPORATION, MISSISSAUGA, ONTARIO	
2.1.2	SLEEVE ANCHOR: PROVIDE SLEEVE ANCHORS OF SIZE SHOWN ON DRAWINGS, INCLUDING MATCHING NUTS AND WASHERS.	
1	FOR DRY LOCATIONS:	
	HLS-3 CARBON STEEL BY HILTI (CANADA) CORPORATION, MISSISSAUGA, ONTARIO	
2	FOR WET OR HIGH HUMIDITY LOCATIONS OR LOCATIONS EXTERIOR TO THE CONDITIONED BUILDING ENVELOPE: HLS-3 STAINLESS STEEL BY HILTI (CANADA) CORPORATION, MISSISSAUGA, ONTARIO	
2.2	ADHESIVE ANCHORS IN DRILLED HOLE	
2.2.1	ANCHOR ROD: PROVIDE ANCHOR RODS OF SIZE, TYPE AND EMBEDMENT LENGTH SHOWN ON DRAWINGS INCLUDING MATCHING NUTS AND MATCHING WASHERS.	
2.2.2	REINFORCING BAR: PROVIDE REINFORCING BAR AS ANCHOR ROD WHERE SPECIFIED ON DRAWING.	
2.2.3	CORROSION PROTECTION: PROVIDE CORROSION PROTECTION SPECIFIED ON DRAWINGS	
2.2.4	ADHESIVE: PROVIDE THE ADHESIVE SPECIFIED ON THE DRAWINGS.	
3	<b>EXECUTION</b>	
3.1	<b>DRILLED-IN ANCHORS</b>	
3.1.1	ARRANGE FOR MANUFACTURER'S TECHNICAL REPRESENTATIVE TO BE PRESENT DURING INSTALLATION OF FIRST	
	FEW ANCHORS OF EACH TYPE. SUBMIT SITE REPORTS BY MANUFACTURER TO CONSULTANT WITHIN ONE WEEK OF EACH VISIT. INDICATE IN REPORTS ANCHOR SIZES AND TYPES INSTALLED, LOCATIONS, AND WHETHER INSTALLATION PROCEDURES WERE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS.	
3.1.2	INSTALL ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS.	
3.1.3	INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER.	
3.1.4	DO NOT DRILL HOLES LARGER IN DIAMETER THAN INDICATED IN MANUFACTURER'S PRINTED INSTRUCTIONS. NOTED ON DRAWINGS.	
3.1.5	PROVIDE MANUFACTURER'S STANDARD EMBEDMENT LENGTH INTO SOLID CONCRETE, UNLESS OTHERWISE NOTED ON DRAWINGS.	
3.1.6	DO NOT CUT REINFORCEMENT TO ACCOMMODATE ANCHORS.	
3.1.7	RELOCATE ANCHORS, AT NO ADDITIONAL COST TO CONTRACT, WHEN OBSTRUCTIONS PREVENT DRILLING HOLES TO REQUIRED DEPTH IN LOCATIONS INDICATED ON DRAWINGS.	
3.1.8	OBTAIN CONSULTANT'S APPROVAL OF NEW LOCATION BEFORE DRILLING HOLE. FILL ABANDONED HOLES WITH SPECIFIED GROUT.	
3.1.9	TIGHTEN EXPANSION ANCHORS USING TORQUE WRENCH. IF FINGER-TIGHT IS REQUIRED, NUT SHOULD BE OFF AFTER TORQUING.	
4	<b>FIELD QUALITY CONTROL</b>	
4.1	ARRANGE FOR INSPECTION AND TESTING COMPANY TO RANDOMLY SELECT AND PULL TEST ANCHORS AS FOLLOWS:	
4.1.1	5% OF EACH TYPE AND SIZE OF ANCHOR INSTALLED ON A WEEKLY BASIS, BUT NOT LESS THAN ONE ANCHOR OF EACH TYPE AND SIZE.	
4.1.2	PULL TEST TO TWICE THE ALLOWABLE DESIGN TENSION CAPACITY OF THE ANCHOR GIVEN BY THE MANUFACTURER.	
4.1.3	SUBMIT REPORTS OF PULL TESTS TO CONSULTANT ON WEEKLY BASIS. INDICATE ON REPORT EACH ANCHOR LOCATION, TEST LOAD AND MODE OF FAILURE, IF APPLICABLE. NOTIFY CONSULTANT IMMEDIATELY IF ANCHOR FAILS PULL TEST.	

ABBREVIATIONS		GN-010CS			
AB	ANCHOR BOLT	Fc	COMPRESSIVE STRENGTH OF CONCRETE	P/C: T/M: V/R	FACTORED LOADS UNFACTORED LOADS
ADD	ADDITIONALLY EXPOSED STRUCTURAL STEEL	FF	FAR FACE	PL	PLATE
AIFB	ASPHALT IMPREGNATED FIBREBOARD	GALV	GALVANIZED	PSL	PARALLEL STRAND LUMBER
ALT	ALTERNATIVE	HB	HIGH BEAM	PVC	POLYVINYL CHLORIDE
AR	ANCHOR ROD	HD	HANGER DOWN	REINF	REINFORCEMENT
ARCH	ARCHITECTURAL	HEF	HORIZONTAL EACH FACE	RTU	ROOF TOP UNIT
B, BOTT	BOTTOM	HH	HOOK-HOOK (HOOK EACH END)	S	SECTION
BC	BOTTOM ELEVATION OF CAISSON	HIF	HORIZONTAL INSIDE FACE	SECT	SECTION
BEW	BOTTOM EACH WAY	HL	HIGH LINE	SF	SPRAY FIRE PROOFED
BL	BOTTOM LOWER LAYER	HOF	HORIZONTAL OUTSIDE	SFD	STEP FOOTING DOWN
BM	BEAM	HOR	HORIZONTAL	SFU	STEP FOOTING UP
BMB	BENDING MOMENT BAR	HSC	HORIZONTALLY SLOTTED CONNECTION	SMR	SMALL
BPL	BEARING-BASE PLATE	HP	HIGH POINT	SJ	STEEL JOIST
BRG	BEARING	HSS	HOLLOW STRUCTURAL SECTION	SL	SLAB
BSMT	BASEMENT	HU	HANGER UP	SOG	SLAB ON GRADE
BUL	BOTTOM UPPER LAYER	ID	INSIDE DIAMETER	SP	SPANDREL, SPRUCE
C	STANDARD CHANNEL	IF	INSIDE FACE	SPF	SPRUCE PINE FR
CA	CENTRE TO CENTRE	KB	KNEE BRACE	ST	STRAIGHT
CAW	COMPLETE WITH COLUMN ABOVE	KN	KILNEWTON	STR	STRIP
CB	COLUMN BELOW	KPa	KILOPASCAL	STRUCT	STRUCTURAL
CANTICANTL	CANTILEVER CONCRETE	KB	KNEE BRACE	T	THICKNESS
CJ	CORNER JOINT	KL	LOWER BEAM	T	TOP
CL	CLEAR	LE	LEFT END	T	TOP ELEVATION OF CAISSON
C.L.	CENTRE LINE	LG	LONG LENGTH	TEW	TOP ELEVATION OF CAP
COMP	COMPOSITE	LL	LOW LINE	TJ	TONGUE AND GROOVE
COL	COLUMN	LL	LONG LEG VERTICAL	TJ	THE JOIST
CONC	CONCRETE	LLV	LONG LEG VERTICAL	TLE	TOP LEFT END
CONN	CONNECTION	LLH	LONG LEG HORIZONTAL	TLL	TOP LOWER LAYER
CONST JT	CONSTRUCTION JOINT CONTINUOUS	LP	LONG POINT	TLL	TOP OF CONCRETE
CONT	CONTINUOUS	LSSJ	LONG SPAN STEEL JOIST	TOS	TOP OF STEEL
DET	DETAIL	LVL	LAMINATED VENEER LUMBER	TRE	TOP RIGHT END
DFR	DOUGLAS FIR	LVL	LAMINATED VENEER LUMBER	TUL	TOP UPPER LAYER
DIA	DIAMETER	LVL	LAMINATED VENEER LUMBER	TYP	TYPICAL
DM	DIMENSION	MAX	MAXIMUM	UB	UPPER BEAM
DL	DEAD LOAD	MC	MOMENT CONNECTION	UIS	UNDERSIDE
DP	DEEP	MIS	MISCELLANEOUS CHANNEL	US	UNDERSIDE OF DECK
DWG	DRAWING	MN	MINIMUM	UL	UPPER LAYER
DWL	DOWEL	m	METRE	UN	UNLESS NOTED OTHERWISE
EA	EACH	m2	SQUARE METRE	UNO	UNLESS NOTED OTHERWISE
ECR	EPOXY COATED REINFORCEMENT	mm	MILLIMETRE	UPT	UPTURNED
EE	EACH END	mm2	SQUARE MILLIMETRE	VB	VERTICAL BRACED
EF	EACH FACE	MECH	MECHANICAL	VF	VERTICAL EACH FACE
EL, EXP, JT	EXPANSION JOINT	MPa	MEGAPASCAL	VEF	VERTICAL INSIDE FACE
EL, ELEV	ELEVATION	NBCC	NATIONAL BUILDING CODE OF CANADA	VIF	VERTICAL OUTSIDE FACE
ELECT	ELECTRICAL	NBC	NOT IN COLUMN BELOW	VOF	VERTICAL OUTSIDE FACE
EMBED	EMBEDMENT	NCF	NOT IN CONTACT	VSC	VERTICAL SLOTTED CONNECTION
EQ	EQUAL	NIC	NOT IN CONTRACT	W	WIDE FLANGE BEAM
EW	EACH WAY	No	NUMBER	WP	WALL PLATE
EX, EXIST	EXISTING	NTS	NOT TO SCALE	WT	STRUCTURAL TEE
FIN	FINISHED	OBC	ONTARIO BUILDING CODE	WWF	WELDED WIRE FABRIC
FL	FLOOR	OC</			



- PARTIAL ROOF FRAMING PLAN**
- DESIGN IS BASED ON A FOLDING PARTITION ASSEMBLY WITH MAXIMUM WEIGHT OF 0.55 kPa.
  - FIELD VERIFY EXISTING CONDITIONS PRIOR TO PREPARATION OF SHOP DRAWINGS AND FABRICATION. REPORT ANY DISCREPANCIES TO CONSULTANT TEAM.
  - RADAR SCAN SOFFIT OF EXISTING SLAB TO CONFIRM LOCATION OF REINFORCING STEEL PRIOR TO FABRICATION. ADJUST HANGER AND BRACE LOCATIONS SO THAT ANCHOR BOLTS AVOID REBAR.
  - REFER TO GENERAL NOTES AND SPECIFICATIONS ON DRAWING S100.

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 2024-11-12  
 L.P. BOWEN  
 100130877  
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**NORTH:**

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Project Manager LB	Drawn
Project Leader KDS	Checked KDS
Client <b>Sunnybrook</b> HEALTH SCIENCES CENTRE 2075 Bayview Ave., Toronto, ON, M4N 3M5	
Project SUNNYBROOK HEALTH SCIENCE CENTRE K WING - HEMATOLOGY	
Drawing Title FRAMING PLAN	
Check Scale (may be photo reduced)	
Project No. ALL-24001907-A0	
Drawing No.	<b>S201</b>