


1. FRAMING AND BRACING:

STEEL COLUMN:		STEEL ANGLE:	
STEEL BEAM/GIRDER:		SECONDARY STEEL:	
OWSJ:		OWSJ BRIDGING:	
STEEL TRUSS:			
STEEL BRACE:			

- A. "A" AXIAL FORCE SUFFIX (E.G. 150A) DENOTES COMPRESSION / TENSION FORCE.
- B. "C" AXIAL FORCE SUFFIX (E.G. 100C) DENOTES COMPRESSION FORCE.
- C. "T" AXIAL FORCE SUFFIX (E.G. 100T) DENOTES TENSION FORCE.
- D. "M" STRONG AXIS MOMENT PREFIX (M100) DENOTES CONNECTION DESIGN MOMENT.

- [illegible]

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- FACTORED CONNECTION SHEAR FORCE
- FACTORED DESIGN MOMENT TO APPLY TO BOTH SIDES OF CONNECTION U.N.O.
- 150
M100
- 50
- 150
M100
- 50

CANT. ——— BEAM CONTINUOUS OVER INTERSECTING BEAM OR COLUMN

-
- EMBEDDED PLATE DESIGNATION
SEE SCHEDULE
- SEE PLAN FOR
CONNECTION FORCES

- STEEL DECK WITHOUT CONCRETE TOPPING:

LINES INDICATE SPAN DIRECTION

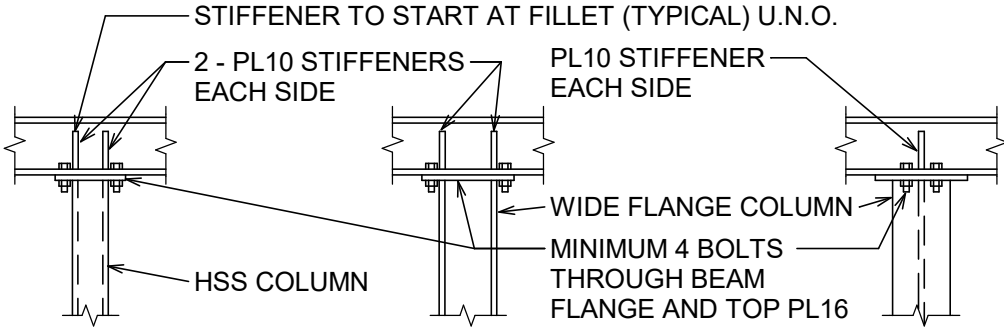
CONCRETE TOPPING ON STEEL DECK:

LINES INDICATE SPAN DIRECTION

SEE PLAN OR SCHEDULES FOR DEPTH OF DECK PROFILE, CONCRETE DEPTH, MINIMUM DECK GAUGE, MINIMUM CONNECTIONS, ETC.

1. READ JONES CHRISTOFFERSEN PROVIDES FIELD REVIEW ONLY FOR THE WORK SHOWN ON THESE STRUCTURAL DRAWINGS. THIS REVIEW IS NOT A "FULL TIME" REVIEW BUT IS CONDUCTED WITH SUCH FREQUENCY AS RJC DEEMS APPROPRIATE TO OBSERVE VARIOUS STAGES OF THE WORK AND TO ASCERTAIN THAT THE WORK IS IN GENERAL IN CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY READ JONES CHRISTOFFERSEN. FIELD REVIEW BY READ JONES CHRISTOFFERSEN IS NOT CARRIED OUT FOR THE CONTRACTOR'S BENEFIT, NOR DOES IT MAKE READ JONES CHRISTOFFERSEN GUARANTORS OF THE CONTRACTOR'S WORK. IT REMAINS THE CONTRACTOR'S RESPONSIBILITY TO BUILD THE WORK IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. RJC SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUB-CONTRACTOR OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. PROVIDE 24 HOURS ADVANCE NOTICE OF EACH REQUIRED FIELD REVIEW. FIELD REVIEWS SHALL BE SCHEDULED TO BE CARRIED OUT DURING NORMAL BUSINESS HOURS UNLESS SPECIAL ARRANGEMENTS ARE MADE WITH RJC.
3. THE WORK TO BE REVIEWED SHALL BE GENERALLY COMPLETE.

23. UNLESS NOTED OTHERWISE ALL CONNECTIONS FOR BEAMS AND GIRDERS SHALL BE DESIGNED FOR A SHEAR BASED ON THE MEMBER'S FULL MOMENT RESISTANCE CAPACITY RELATED TO A UNIFORM LOAD ON A SIMPLE SUPPORTED SPAN.
24. TOP FLANGES OF BEAMS TO BE FREE OF ALL PAINT, DIRT, HEAVY RUST, MILL SCALE, SAND AND OTHER MATERIALS WHICH WILL INTERFERE WITH WELDING OF STUD SHEAR CONNECTIONS AND STEEL DECK TO BEAMS.
25. BEAMS NOTED AS COMPOSITE ON THE DRAWINGS REQUIRE STUD SHEAR CONNECTIONS, SEE ALSO SHEAR CONNECTOR NOTES. SEE ALSO PLANS, SECTIONS, DETAILS AND SCHEDULES FOR STUDS SHOWN ON BEAMS/GIRDERS/DAG-STRUTS ETC. OTHER THAN COMPOSITE BEAMS.
26. UNLESS NOTED OTHERWISE WHERE BEAMS SIT OVER COLUMNS, PROVIDE FULL WIDTH PL10 STIFFENER EACH SIDE OVER COLUMN. FULL HEIGHT STIFFENERS REQUIRED WHERE A COLUMN IS OVER THE BEAM OR WHERE NOTED ON PLANS AND DETAILS.



STIFFENER PLATE EACH SIDE OF BEAM WEB WITH MINIMUM THICKNESS OF SIDE PLATES OR COLUMN BELOW FLANGE, BUT NOT LESS THAN 13mm THICK

Q COL

BEAM PER PLAN

25

TYP.

100 MIN.

2X COLUMN DEPTH

CAP PLATE WITH WIDTH MIN. 50mm GREATER THAN COLUMN SIZE OR EQUAL TO BEAM FLANGE WHICHEVER IS GREATER

TYP.

CONNECT TO COLUMN FOR CAPACITY OF PLATES

SIDE PLATES WITH MINIMUM THICKNESS OF COLUMN FLANGE, BUT NOT LESS THAN 12mm THICK WHEN COLUMN IS PERPENDICULAR TO BEAM WEB. CONNECT TO COLUMN FOR CAPACITY OF PLATES. MINIMUM TOTAL SIDE PLATE AREA EQUAL TO $1/2$ COLUMN AREA

Diagram illustrating a beam-to-column connection detail. The diagram shows a cross-section of a beam (labeled "BEAM PER PLAN") and a column. A stiffener plate is attached to the column web, with the text "STIFFENER PLATE EACH SIDE OF BEAM WEB (SEE ADDITIONAL NOTES ABOVE)". The stiffener plate is connected to the column web using bolts. The column web is labeled "COL". The beam flange is labeled "BEAM PER PLAN". The stiffener plate is labeled "STIFFENER PLATE". The connection is shown with a "CAP PLATE WITH WIDTH MIN. 50mm GREATER THAN COLUMN SIZE OR EQUAL TO BEAM FLANGE WHICHEVER IS GREATER". The cap plate is shown with a "TYP." (typical) symbol.

STIFFENER PLATE EACH SIDE OF BEAM WEB WITH MINIMUM THICKNESS OF BEAM WEB BELOW, BUT NOT LESS THAN 13mm THICK

G COL

BEAM PER PLAN

CAP PLATE WITH WIDTH MIN. 50mm GREATER THAN COLUMN SIZE OR EQUAL TO BEAM FLANGE WHICHEVER IS GREATER

TYP.

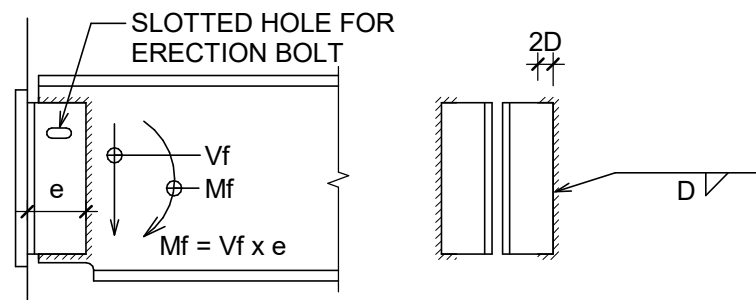
NOTE: FOR LIGHTLY LOADED COLUMNS WHERE ($C_f < 100 \text{ kN}$) COLUMN WEB IS PERPENDICULAR TO BEAM WEB, ONE (1) BEAM STIFFENER PLATE CAN BE USED CENTERED ON COLUMN WEB.

1. FABRICATION, ERECTION, STRUCTURAL DESIGN, AND DETAILING OF ALL STEEL SHALL BE IN ACCORDANCE WITH CSA S16.
2. PRIOR TO SUBMITTING SHOP DRAWINGS THE CONTRACTOR SHALL NOTIFY RJC IN WRITING THAT THE FABRICATOR IS CERTIFIED TO A MINIMUM OF DIVISION 2 OF CSA W47.1.
3. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO START OF STEEL FABRICATION. ALSO REFER TO "SHOP DRAWINGS" NOTE IN THE GENERAL NOTES SECTION OF THE STRUCTURAL DRAWINGS.
4. FILLET WELDS SHALL BE 5 mm MINIMUM UNLESS NOTED OTHERWISE.
5. BOLTS SHALL BE 3/4" MINIMUM A325 UNLESS NOTED OTHERWISE.
6. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS IN EACH CONNECTED PIECE AND BE DESIGNED AS BEARING CONNECTIONS, U.N.O.
7. UNLESS NOTED OTHERWISE, BOLTED CONNECTIONS WITH OVERSIZED OF SLOTTED HOLES SHALL HAVE WASHERS PER CISC STANDARD PRACTICE:
 - A. OVERSIZED AND SHORT SLOTS: HARDENED WASHERS.
 - B. LONG SLOTS: PLATE WASHERS FULLY COVERING THE SLOTS.
8. IN ADDITION TO ALL OTHER CRITERIA SPECIFIED IN ASTM F1554, ALL HOOKED ANCHOR RODS IN CONCRETE SHALL BE MANUFACTURED WITH A MINIMUM INSIDE BEND RADIUS OF 3 TIMES THE ROD DIAMETER, UNLESS NOTED OTHERWISE.
9. ALL WELDED HEADED STUDS AND WELDED DEFORMED BAR ANCHORS SHALL BE INSTALLED AS PER THE MANUFACTURERS SPECIFICATIONS AND RECOMMENDATIONS OR SHOP FILLET WELDED TO DEVELOP THE TENSILE FACTORED RESISTANCE OF THE BAR. ANY FIELD FILLET WELDED DEFORMED BARS OR STUDS WILL BE REJECTED. SEE PLANS, SECTIONS, DETAILS, AND SCHEDULES FOR LOCATIONS ETC., THE CONTRACTOR SHALL CO-ORDINATE THE DESIGN, SUPPLY, AND INSTALLATION OF ALL STUDS AND ANCHORS, INCLUDING, BUT NOT LIMITED TO STUDS AND DEFORMED BAR ANCHORS ON COMPOSITE BEAMS, DRAG STRUTS, EMBEDDED PLATES, ETC.

10. UNLESS NOTED OTHERWISE, COLUMN CAP PLATES SHALL BE 16 mm THICK AND COLUMN BASE PLATES SHALL BE 20 mm MINIMUM THICK.
11. PROVIDE 6 mm CAP PLATES FOR ALL HSS MEMBERS U.N.O.
12. CONNECTION DETAILS SHOWN ON THE STRUCTURAL DRAWINGS SHALL NOT BE ALTERED BY THE CONTRACTOR WITHOUT WRITTEN APPROVAL FROM READ JONES CHRISTOFFERSEN LTD.
13. UNLESS NOTED OTHERWISE ON THE PLANS, REFER TO THE DETAILS IN THE GENERAL NOTES FOR FRAMING FOR SUPPORT OF ROOF TOP MECHANICAL EQUIPMENT.
14. STEEL SHALL BE PREPARED AND FINISHED IN ACCORDANCE WITH CSA S16 AND THE ARCHITECTURAL DRAWINGS AND PAINTING SPECIFICATIONS WHICH MAY INCLUDE ADDITIONAL CLEANING AND PRIMING REQUIREMENTS.
15. ALL STRUCTURAL STEEL OUTSIDE OF THE BUILDING ENVELOPE TO BE HOT-DIP GALVANIZED UNLESS NOTED OTHERWISE.
16. DESIGN DRAWINGS INCLUDE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. SEE ALSO ARCHITECTURAL DRAWINGS FOR ROOF AND FLOOR ELEVATIONS, ROOF SLOPES, EDGE DETAILS, AND ADDITIONAL DIMENSIONS AND DETAILS. WHERE ELEVATIONS, ROOF SLOPES, ETC., ARE SHOWN ON THE STRUCTURAL DRAWINGS, THEY MUST BE CONFIRMED WITH THE ARCHITECTURAL DRAWINGS.
17. UNLESS INDICATED ON APPROVED SHOP DRAWINGS OR OTHERWISE APPROVED IN WRITING BY RJC, DO NOT CONSTRUCT REINFORCED OR UN REINFORCED SLEEVES THROUGH STEEL BEAMS
18. UNLESS NOTED OTHERWISE, DO NOT OVERSIZE HOLES IN STEEL TO FIT ANY CAST-IN-PLACE OR POST-INSTALLED ANCHORS WITHOUT APPROVAL IN WRITING BY RJC.
19. UNLESS NOTED OTHERWISE, CAST-IN-PLACE ANCHOR RODS FOR COLUMN BASES TO HAVE PLACEMENT TOLERANCE PER CSA A23.1 AND CISC CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL. ANCHOR ROD HOLE SIZES IN STEEL PLATES NOT TO EXCEED DIAMETER OF FASTENER + 6 mm.
20. UNLESS NOTED OTHERWISE, HOLE SIZES IN STEEL PLATES FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH CSA S16 REQUIREMENTS FOR STANDARD DIAMETER HOLES IN BOLTED STEEL CONNECTIONS;

[ALSO REFER TO "POST-INSTALLED MECHANICAL AND ADHESIVE ANCHOR" NOTES AND "RENOVATION" NOTES.]

- A. WHERE CONNECTION FORCES ARE NOT SHOWN ON THE DRAWINGS THE CONNECTION DESIGN SHALL SATISFY THE REQUIREMENTS OF CSA S16 - CLAUSE 27.
 - B. THE BEAM TO GIRDER DISSIPATION SYSTEM SHALL SATISFY THE LIMITS OF $F_u F_y$ AND CHARPY V-NOTCH IMPACT REQUIREMENTS AS NOTED IN CSA S16 - CLAUSE 27.1.5.
 - C. WELDS AND WELD MATERIAL SHALL SATISFY CSA S16 - CLAUSE 27.1.5.3 (CHARPY REACTION WELDS).
 - D. BOLTED CONNECTIONS SHALL SATISFY CSA S16 - CLAUSE 27.1.6.
- UNLESS NOTED, BEAMS AND GIRDER SHEAR CONNECTIONS TO EMBEDDED PLATES SHALL BE DOUBLE ANGLE FRAMING CONNECTIONS WELDED TO THE BEAM WEB THUS:



~ CONTINUED ~

1. THE COMPLETED RENOVATION STRUCTURE SHOWN ON THE STRUCTURAL DRAWINGS HAS BEEN DESIGNED IN SUBSTANTIAL ACCORDANCE WITH THE ONTARIO BUILDING CODE 2012 O.REG 88/19 AND SUBSEQUENT REVISIONS WHICH IS BASED ON THE NATIONAL BUILDING CODE OF CANADA 2015.

1. AS PART OF OUR CONSTRUCTION PHASE SERVICES, RJC WILL REVIEW SHOP DRAWINGS PERTAINING TO WORK SHOWN ON R/C'S DRAWINGS BY MEANS OF APPROPRIATE RATIONAL SAMPLING PROCEDURES AND COMMENT ON THE ACCURACY WITH WHICH THE CONTRACTOR PREPARED THE DRAWINGS.
2. REVIEW OF SHOP DRAWINGS IS FOR THE SOLE PURPOSE OF ASCERTAINING CONFORMANCE WITH THE GENERAL DESIGN CONCEPT AND IS NOT AN APPROVAL OF THE DETAILED DESIGN INHERENT IN THE SHOP DRAWINGS. RESPONSIBILITY FOR WHICH SHALL REMAIN WITH THE CONTRACTOR SUBMITTING THEM. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMISSIONS IN THE SHOP DRAWINGS AND FOR MEETING ALL REQUIREMENTS OF THE CONTRACT DRAWINGS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INFORMATION PERTAINING TO THE FABRICATION PROCESS, TECHNIQUES FOR CONSTRUCTION AND INSTALLATION, AND FOR CO-ORDINATION OF THE WORK OF ALL SUB-TRADES.
3. SHOP DRAWINGS SHALL BE COMPLETE AND INCLUDE ANY REQUIRED SEALS FROM A PROFESSIONAL ENGINEER REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED PRIOR TO SUBMISSION.

STRUCTURAL STEEL SECTIONS SHALL BE NEW AND CONFORM TO THE FOLLOWING U.N.O.:

A.	W AND WT SHAPES	CSA G40.21 GRADE 345WM
		ASTM A992
B.	C, L, M, MC, MT, S, AND ST SHAPES	CSA G40.21 CSA G40.21 350W
		ASTM A992, OR
		ASTM A572 GRADE 50
C.	HP SHAPES	ASTM A572 GRADE 50
D.	RECTANGULAR OR SQUARE HSS	ASTM A500 GRADE C,
		CSA G40.21 GRADE 350W,
		OR ASTM A1085
		CLASS C OR ASTM A1085
E.	ROUND HSS	ASTM A500 GRADE C
F.	PIPE	ASTM A53 GRADE B
G.	ROLLED PLATES AND BARS	CSA G40.21 GRADE 300W
H.	WWF AND WRF SHAPES	CSA G40.21 GRADE 350W
J.	BOLTS (SEE PLANS AND DETAILS)	ASTM F3125 GRADE A325
		OR A490
K.	STRUCTURAL STEEL ANCHOR RODS	ASTM F1554
	(UNLESS NOTED OTHERWISE)	GRADE 36 MINIMUM
L.	REINFORCING BAR ANCHOR BOLTS	CSA G30.18 GRADE 400R

2. USE OF STRUCTURAL STEEL SHAPES, PLATE OR BARS WITH HIGHER GRADE THAN NOTED ABOVE SUBJECT TO APPROVAL BY RJC.

3. ALL ASTM F1554 ANCHOR RODS SHALL HAVE SHOP-APPLIED COLOUR MARKING TO FACILITATE IDENTIFICATION OF GRADE IN FIELD:
GRADE 36 = BLUE; GRADE 55 = YELLOW; GRADE 105 = RED.

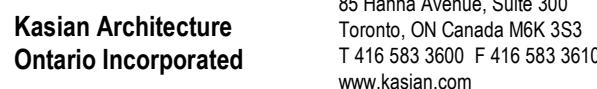
4. DESIGN FORCES INDICATED ON DRAWINGS FOR STRUCTURAL STEEL WORK ARE FACTORED FORCES UNLESS NOTED OTHERWISE.
FORCES ARE VERTICAL SHEAR FORCES UNLESS NOTED OTHERWISE.

METRIC

A.	FORCES	kN
B.	MOMENTS	kN-m
C.	LINE LOADS	kN/m
D.	DISTRIBUTED LOADS	kPa

SEE "DESIGN LOADS" NOTES FOR DEFINITIONS AND VALUES OF LIVE LOAD, DEAD LOAD AND SUPERIMPOSED DEAD LOAD. SEE ALSO PLANS FOR OTHER LOAD/FORCE REQUIREMENTS.

1. ALL CONNECTIONS TO BE DESIGNED BY FABRICATOR UNLESS NOTED OTHERWISE. ALL BEAM CONNECTIONS TO BE STANDARD FRAME BEAM CONNECTIONS OR EQUIVALENT, UNLESS NOTED OTHERWISE. THE FABRICATOR SHALL SUBMIT SUMMARY DESIGN DRAWINGS FOR REVIEW SHOWING IN DETAIL THE "STANDARD" CONNECTIONS AND THEIR CAPACITIES THAT IS INTENDED FOR USE ON THE PROJECT. THESE DRAWINGS ARE IN ADDITION TO THE REGULAR SHOP DRAWINGS, AND SHALL PRECEDE THEM.
2. SHOP DRAWINGS SHALL BE PREPARED UNDER THE DIRECTION OF A SPECIALTY STRUCTURAL ENGINEER. FOR THOSE CONNECTIONS AND COMPONENTS DESIGNED BY THE FABRICATOR, THIS ENGINEER OR THEIR REPRESENTATIVE SHALL VISIT THE SITE TO REVIEW IN PLACE THE CONNECTIONS AND COMPONENTS DESIGNED BY THIS ENGINEER TO SATISFY THEMSELVES THAT THESE CONNECTIONS AND COMPONENTS SUBSTANTIALLY COMPLY WITH THEIR DESIGN ON THE SHOP DRAWINGS. THIS ENGINEER SHALL PROVIDE A LETTER TO RJC TO THIS EFFECT. THIS ENGINEER SHALL ALSO PROVIDE SEALED SKETCHES FOR ALL FIELD MODIFICATIONS MADE TO THEIR DESIGN.
3. THE CONTRACTOR SHALL NOTIFY THE CONSULTANT IN WRITING (AND BEFORE THE SUBMISSION OF SHOP DRAWINGS) AS TO WHO THE ENGINEER WILL BE THAT WILL BE DESIGNING AND PROVIDING FIELD REVIEW FOR THE CONNECTIONS AND COMPONENTS DESIGNED BY THE CONTRACTOR.
4. DRAWINGS OF COMPONENTS AND CONNECTIONS DESIGNED BY THE FABRICATOR'S SPECIALTY STRUCTURAL ENGINEER SHALL BE SIGNED AND SEALED BY THIS ENGINEER OR A LETTER SHALL BE SUBMITTED AT THE TIME THE SHOP DRAWING PRODUCTION SIGNED AND SEALED BY THIS ENGINEER, IDENTIFYING WHAT WAS DESIGNED AND LISTING THE FINAL DRAWINGS WITH DATAS AND REVISION NUMBERS.
5. DRAWINGS AND SPLICES NOT SHOWN ON THE STRUCTURAL CONNECTIONS BUT REQUESTED BY THE FABRICATOR MUST BE ACCEPTABLE TO THE ENGINEER AND ON THE SHOP DRAWINGS. TESTING OF THESE CONNECTIONS SHALL BE AT THE DISCRETION OF RJC AND TO THE CONTRACTORS ACCOUNT.



6	2024-04-17	ISSUED FOR TENDER		AV
5	2024-04-17	ISSUED FOR BUILDING PERMIT APPLICATION		AV
4	2023-12-06	ISSUED FOR MOH 2/3 SUBMISSION		JMW
3	2023-10-02	ISSUED FOR 100% CD		JMW
2	2023-08-14	ISSUED FOR 90% CD		JMW
1	2023-07-14	ISSUED FOR 60% CD		JMW
REV.	YYYY-MM-DD	REVISION / DRAWING ISSUE		REVIEW



Read Jones Christoffersen Ltd
Engineers
rjc.ca

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PROJECT

SHN DIALYSIS CARE UNIT
CENTENARY HOSPITAL

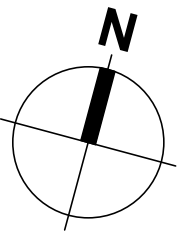
SCARBOROUGH, ONTARIO CANADA

GENERAL NOTES AND TYPICAL DETAILS

DRAWING ISSUE

ISSUED FOR TENDER

PROJECT NO. TOR.119419.0007	PLOT DATE APR. 17, 2024	DRAWN MSS
SCALE As indicated @ ARCH D	REVIEWED JMW	
DRAWING NO. S100		REVISION 6

CONSULTANT

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PROJECT

SCARBOROUGH, ONTARIO CANADA

DRAWING TITLE

PART ROOF PLAN AND SECTIONS

DRAWING ISSUE

ISSUED FOR TENDER

DRAWING NO.	REVISION
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6



1 **TYPICAL POST AND BASEPLATE DETAIL**
S200 1 : 20



NOTES:

1. REFER TO ARCHITECTURAL FOR ALL DIMENSIONS AND ELEVATIONS. TOP OF STRUCTURAL STEEL TO BE AT ELEVATION OF 0 mm BELOW FINISH FLOOR ELEVATION. EXCEPT AS CROSSED AND NOTED ON PLAN.
2. CONTRACTOR TO CHECK X-RAY EXISTING SLAB PRIOR TO DRILLING ANCHORS. SEND SCAN RESULTS TO RJC PRIOR TO DRILLING. DO NOT CUT ANY EXISTING REINFORCING.
3. REFER TO AHU MECHANICAL DRAWINGS FOR ATTACHMENT OF AHU TO ELEVATED STRUCTURAL FRAME.
4. CONTRACTOR TO PROVIDE PRIOR CORKING LOCATION PLAN TO CONSULTANT FOR REVIEW PRIOR TO DRILLING. ALL LOCATIONS OF CORES TO BE SCANNED TO LOCATE ALL REINFORCING AND TO BE REVIEWED BY STRUCTURAL ENGINEER PRIOR TO DRILLING. NO CORES PERMITTED THROUGH BEAMS OR DROP PANELS OR WITHIN 1200mm OF COLUMN.
5. ALL STRUCTURAL STEEL MEMBERS MUST BE CSA G40 21 350W GRADE.
6. ALL STRUCTURAL STEEL TO BE HOT-DIP GALVANIZED.

DESIGN ALL BEAM CONNECTIONS FOR THE FACTORED FORCES NOTED ON PLAN. WHERE NO FORCES ARE SHOWN DESIGN SHEAR CONNECTIONS FOR A MINIMUM OF 75KN

DESIGN ALL BEAM CONNECTIONS BETWEEN GIRDERS AND STRUCTURAL COLUMNS FOR THE AXIAL LOADS NOTED ON PLAN. WHERE NO FORCES ARE SHOWN, DESIGN CONNECTIONS FOR THE AXIAL LOAD OF $\pm 75KN$ IN ADDITION TO ANY NOTED FACTORED FORCES.

3. PRIOR TO FABRICATION OF STEEL SUBMIT DRAWINGS SHOWING THE LOCATION AND SIZE OF ALL OPENINGS OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS FOR REVIEW.

10. FORCES SHOWN ON ELEVATION ARE AS FOLLOWS:

- Af = FACTORED AXIAL FORCE (kN)
- Cf = FACTORED COMPRESSION FORCE (kN)
- Tf = FACTORED TENSION FORCE (kN)
- Vf = FACTORED SHEAR FORCE (kN)
- Mf = FACTORED MOMENT (kNm)
- Trf = FACTORED TORSION (kNm)